

Anchor Channels PEC-TA hot-rolled

Technical Datasheet December 2020







Selector for anchor channels PEC-TA hot-rolled

Ancho	r channels PEC-TA hot-rolled										
Type		PEC-TA-	CE 40/22	PEC-TA-	CE 50/30	PEC-TA-CE 52/34					
				C,							
Chann	el bolt type	HBC-40/22	HBC-40/22-N	HBC-50/30	HBC-50/30-N	HBC-50/30	HBC-50/30-N				
Channel bolt size		M12-M16	M16	M12-M20	M16-M20	M12-M20	M16-M20				
	Cracked concrete			•		•					
erial	Uncracked concrete										
Base material	Normal weight concrete (NWC)	•	•		•		•				
Base	Light weight concrete (LWC)										
	Reinforced/unreinforced			•		•					
	European Technical Assessment (ETA)			•							
ta	Static/quasi-static 2D			•	•						
al da	Static 3D (only HDG)	-		-		-					
Technical data	Seismic	-	-	-	-	-	-				
Te	Fatigue (only HDG)	-	-	-	-		-				
	Fire		-	•	-	•	-				
u	Hot-dip galvanized (HDG)										
icatic	Stainless steel A4	•	-		-		-				
Specification	Tear-out band	•	~	✓	~	•	~				
S.	End caps	•	✓	✓	~	✓	✓				
PROFIS	S Anchor Channel software				/						

ETA approved Internal tests

Free Design-Software

For your individual design we suggest to use our user-friendly design software. PROFIS Anchor Channel is a reliable planning tool to optimize the design of anchor channels for each specific construction project. A user-friendly interface allows quick and easy selection of suitable anchor channels and bolts for any type of application.

The calculations are based on the current design code EN 1992-4/EOTA-TR047 and the European Technical Assessment ETA-16/0929.

Benefits



- Fast and efficient planning of fixings with anchor channels
- Clear and concise calculation reports
- Wide range of design parameters
- Functions for automatic planning optimization

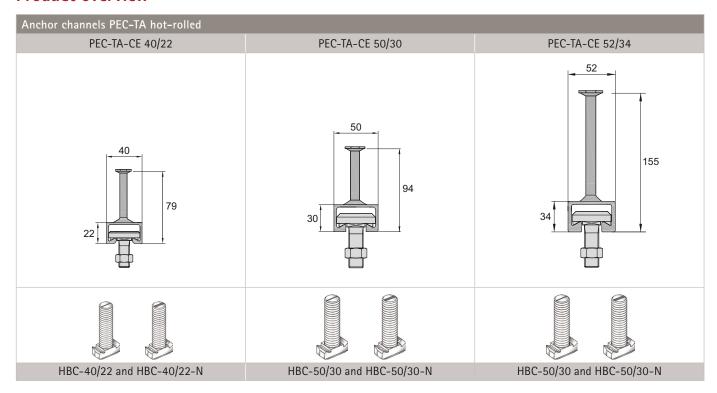
Download PROFIS Anchor Channel Software here:

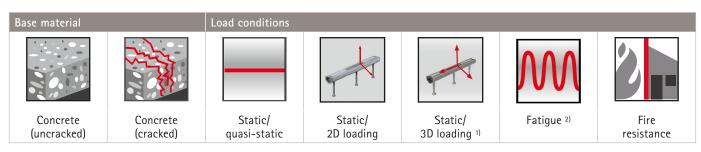
www.pec-europe.com/en/downloads/software.html

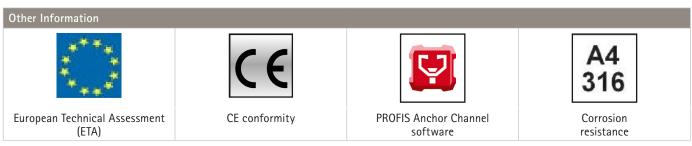


Product range

Product overview







Approvals

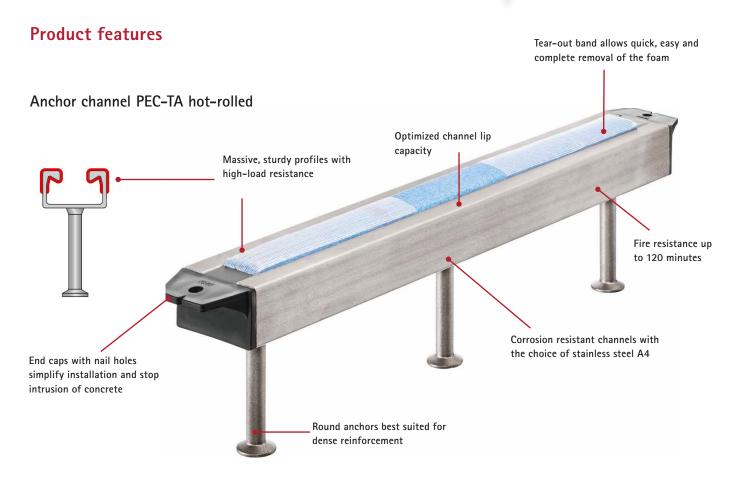
Description	Issuing Authority	No.
European Technical Assessment (ETA) covering 2D, 3D static, fatigue and fire loads	DIBt Berlin	ETA-16/0929

^{1) 52/34} anchor channel is ETA approved for 3D static load. 3D load for PEC-TA 40/22 and PEC-TA 50/30 based on internal tests

²⁾ 52/34 anchor channel is ETA approved for fatigue loads



Product features



Nomenclature of anchor channels PEC-TA hot-rolled

• PEC anchor channel	2 Profile type and size		4 Foam filler	• Anchor channel length [mm]
PEC-TA-CE	40/22	Hot-dip galvanized (FV) or A4 (stainless steel)	PKF	300

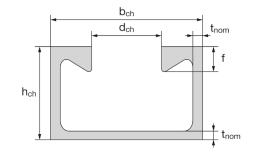
Examples: ① Channel type ② Profile type/size ③ Material finish ④ PKF ⑤ Length





Channel dimensions hot-rolled profiles

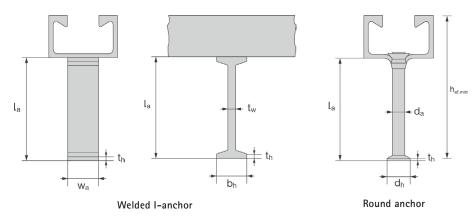
Anchor channel	b _{ch}	h _{ch}	t _{nom}	d _{ch}	f	l _y
			[mm]			[mm ⁴]
PEC-TA-CE 40/22	40.1	23.0	2.7	18.0	6.0	21.504
PEC-TA-CE 50/30	49.6	30.0	3.2	22.5	8.1	57.781
PEC-TA-CE 52/34	52.5	34.0	4.0	22.5	11.5	97.606



Anchor dimensions (welded I-anchor or round anchor)

Anchor channel		I-anchor				Round anchor					
	min l _a	t _w	b _h	t _h	Wa	A_h	min l _a	d _a	d _h	t _h	A_h
			[mm]			[mm ²]		[m	m]		[mm ²]
PEC-TA-CE 40/22	62.0	5.0	20.0	5.0	20.0	300	58.0	8.0	16.0	2.0	151
PEC-TA-CE 50/30	69.0	5.0	20.0	5.0	25.0	375	66.0	10.0	20.0	2.2	236
PEC-TA-CE 52/34	125.0	6.0	25.0	5.0	40.0	760	123.5	11.0	24.3	2.5	369

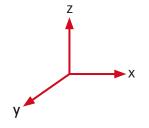
Anchor types



Installation parameters for anchor channels

PEC-TA-CE	40/22	50/30	52/34		
inimum effective embedment depth h _{ef,min}			79	94	155
Minimum anchor spacing	S _{min}				
Maximum anchor spacing	S _{max}		250		
End spacing	X	[mm]	25 ¹⁾		35 ²⁾
Minimum channel length	I _{min}		150	150	170 ³⁾
Minimum edge distance (c ₁₁ , c _{1,2} & c ₂₁ , c ₂₂)	C _{min}		50	75	100
Minimum thickness of concrete member	h _{min}		100	105	165

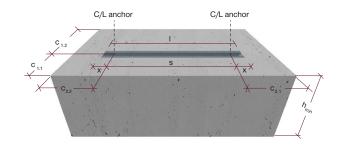
 $^{^{1)}\,}$ The end spacing may be increased from 25 to 35 mm $^{2)}\,$ x=25 mm for welded I-anchors



X - Longitudinal shear -V_x

Y - Perpendicular shear -V_y

Z - Tension -N



 $^{^{3)}}$ $I_{min} = 150$ mm for welded I-anchors



Material of anchor channels and channel bolts

Component		Carbon steel				
	Mechanical properties	Coa	ting	Mechanical properties		
1	2a	2b	2c	3		
Channel profile	1.0038, 1.0044, 1.0045 according to EN 10025: 2005 1.0976, 1.0979 according to EN 10149: 2013	Hot-dip galvar accord EN ISO 10684:	ling to	1.4362, 1.4401 1.4404, 1.4571, 1.4578 according to EN 10088: 2005		
Anchor	1.0038, 1.0213, 1.0214 according to EN 10025: 2005 1.5523, 1.5535 according to EN 10263: 2002-02	-	Hot-dip galvanized ≥ 50 μm according to EN ISO 10684: 2004/ AC: 2009	1.4362, 1.4401 1.4404, 1.4571, 1.4578 according to EN 10088: 2005 ³⁾		
Channel bolt	Steel grade 4.6 and 8.8 according to EN ISO 898-1: 2013	Electroplated according to EN ISO 4042: 1999	Hot-dip galvanized ≥ 50 μm according to EN ISO 10684: 2004/ AC: 2009	Grade 50 or 70 according to EN ISO 3506: 2009		
Plain washer ¹⁾ according to ISO 7089: 2000 and ISO 7093-1: 2000	Hardness class A ≥ 200 HV	Electroplated according to EN ISO 4042: 1999	Hot-dip galvanized ≥ 50 μm according to EN ISO 10684: 2004/ AC: 2009	1.4401, 1.4404 1.4571, 1.4578 according to EN 10088: 2005		
Hexagonal nut according to ISO 4032: 2012 or DIN 934: 1987-10 ²⁾	Property class 5 or 8 according to EN ISO 898-2: 2012	Electroplated according to EN ISO 4042: 1999	Hot-dip galvanized ≥ 50 μm according to EN ISO 10684: 2004/ AC: 2009	Property class 50, 70 or 80 according to EN ISO 3506: 2009		

Channel bolts nomenclature

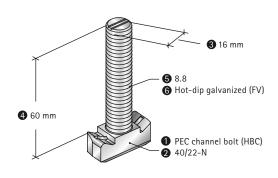
• PEC channel bolt	2 Bolt type	3 Diameter	4 Bolt length [mm]	5 Steel grade	6 Finish or material
НВС	40/22	M12	50	8.8 & A4-70	FV (HDG) & A4 (stainless steel)
НВС	40/22-N	M16	60	8.8	Steel (8.8)

Examples: ① Channel bolt ② Bolt type ③ Diameter ④ Bolt length ⑤ Steel grade ⑥ Finish or material

HBC-40/22 M12x50 8.8F (standard bolt)

3 12 mm **6** 8.8 6 Hot-dip galvanized (FV) and stainless steel (A4) **4** 50 mm PEC channel bolt (HBC) **2** 40/22

HBC-40/22-N M16x60 8.8F (notched bolt)



¹⁾ In scope of delivery only for notched bolts 2) Hexagonal nuts according to DIN 934 :1987-10 for channel bolts made from carbon steel (4.6) and stainless steel

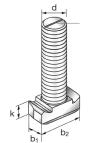
³⁾ Anchors made of carbon steel according column 2a may also be used if they are welded and their concrete cover is more than 50mm and the tempering colors are removed



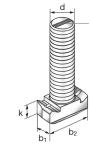
Channel bolt dimensions

Anchor channel	Channel bolt type	Dimensions					
		b ₁	b ₂	k	d		
			[m	m]			
	HBC-40/22 14.0 17.0 HBC-40/22-N 17.0	14.0		10.5	10		
PEC-TA-CE 40/22			33.0	11.5	12		
		17.0		11.5	16		
		33.0	11.5	16			
		17.0		14.5	12		
DEO TA OF FOLOO	HBC-50/30	17.0	42.0	15.5	16		
PEC-TA-CE 50/30 PEC-TA-CE 52/34		21.0		15.5	20		
1 LC 111 CL 32/34	HPC EO/20 N	01.0	42.0	15.5	16		
	HBC-50/30-N	21.0	42.0	15.5	20		

Channel bolts marking



Groove for marking the position of standard bolts



Double groove for marking the position of notched bolts

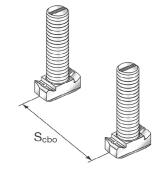
HBC-40/22, HBC-50/30

HBC-40/22-N, HBC-50/30-N

Minimum spacing for channel bolts

Channel bolt			M10	M12	M16	M20
Minimum spacing between channel bolts	S _{cbo,min}	[mm]	50	60	80	100

 s_{cbo} = center to center spacing between channel bolts ($s_{cbo,min}$ = 5d)



Steel grade and corrosion protection

Channel bolt	Carbon	steel 1)	Stainless steel ¹⁾		
Steel grade	4.6	8.8	A4-50	A4-70	
f _{uk} [N/mm ²]	400	800 / 830 ²⁾	500	700	
f _{yk} [N/mm ²]	240	640 / 660 ²⁾	210	450	
Corrosion protection		3) 4)	R	5)	

- 1) Material properties according to table on page 7
- ²⁾ Material properties according to EN ISO 898-1 :2013
- 3) Electroplated

- 4) Hot-dip galvanized
- 5) Stainless steel

Definition of load directions in the following tables

Tension load (N)

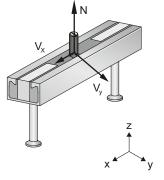
z-direction (in direction of channel bolt)

Shear load (V_y)

y-direction (perpendicular to longitudinal axis of the channel)

Longitudinal shear load (V_x)

x-direction (in direction of longitudinal axis of the channel)





Steel resistance values – static resistance under tension, perpendicular shear and longitudinal shear load



Static/ quasi-static

Steel resistance values under tension load - anchor channel

PEC-TA-	·CE		40/	22	50)/30	52/34		
Steel fai	lure: Failure of anchor		I	R	I	R	1	R	
	Characteristic resistance	$N_{Rk,s,a}$ [kN]	20.	0	31.0		55.0		
	Design resistance	N _{Rd,s,a} [kN]	11.1		17.2		30.6		
Steel fai	lure: Failure of connection bet	ween anchor and chai	nnel						
	Characteristic resistance	N _{Rk,s,c} [kN]	20.0		31.0		55.0		
1	Design resistance	N _{Rd,s,c} [kN]	11.	11.1		17.2		0.6	
Steel fai	lure: Local failure by flexure of	channel lips							
•	Characteristic or design spacing of the channel bolts	S _{I,N} [mm]	79)	Ś	98	10	05	
	Characteristic resistance	N ^o _{Rk,s,I} [kN]	47.	47.9		50.5		65.0	
	Design resistance	N ^o _{Rd,s,l} [kN]	26.	26.6		28.1		5.1	

Steel resistance values under tension load - channel

PEC-TA-CE		40/22	50/30	52/34	
Steel failure: Fail	ure by flexure of channel				
	Characteristic flexural resistance	M _{Rk,s,flex} [Nm]	1013	2084	3435
	Design flexural resistance	M _{Rd,s,flex} [Nm]	881	1812	2987

Displacements under tension load

PEC-TA-CE	40/22	50/30	52/34	
Tension load	N [kN]	13.9	14.3	25.8
Short-term displacement 1)	δ_{N0} [mm]	2.3	2.2	1.4
Long-term displacement 1)	$\delta_{\text{N}\infty}\;[\text{mm}]$	4.6	4.4	2.8

¹⁾ Displacements in midspan of the anchor channel, including slip of channel bolt, deformation of channel lips, bending of the channel and slip of the anchor channel in concrete





Steel resistance values under perpendicular shear load - anchor channel

Static/ quasi-static

					4
PEC-TA-C	E		40/22	50/30	52/34
Steel failu	re: Failure of anchor				
	Characteristic resistance	V _{Rk,s,a,y} [kN]	26.0	40.3	121.5
→	Characteristic resistance	$V_{Rk,s,a,x}$ [kN]	12.1 1)	18.9 ¹⁾	33.1
	Decian recistance	$V_{Rd,s,a,y}$ [kN]	17.3	26.9	81.0
	Design resistance	$V_{Rd,s,a,x}$ [kN]	8.1	12.6	22.1
Steel failu	re: Failure of connection betwe	en anchor and cha	nnel		
	Characteristic resistance	V _{Rk,s,c,y} [kN]	26.0	40.3	121.5
→		V _{Rk,s,c,x} [kN]	12.0 1)	18.6 ¹⁾	28.1
	Design resistance	V _{Rd,s,c,y} [kN]	14.4	22.4	67.5
	Design resistance	V _{Rd,s,c,x} [kN]	6.7	10.3	15.6
Steel failu	re: Local failure by flexure of c	hannel lips			
→	Characteristic or design spacing of the channel bolts for shear	S _{I,V} [mm]	80.0	99.0	105.0
	Characteristic resistance	V ⁰ _{Rk,s,l,y} [kN]	55.0	91.7	71.5
	Design resistance	V ⁰ _{Rd,s,l,y} [kN]	30.6	50.9	39.7

¹⁾ Internal technical data

Steel resistance values for local flexure of channel lips under longitudinal shear load

PEC-TA-CE				40/22 ¹⁾	50/30 ¹⁾	52/34
		V ⁰ _{Rk,s,l,x} [kN]	HBC-40/22-N M16 8.8F	3.0	-	-
	Characteristic resistance		HBC-50/30-N M16 8.8F	_	6.0	8.3
			HBC-50/30-N M20 8.8F	-	6.0	8.3
			HBC-40/22-N M16 8.8F	1.7	-	-
		V ⁰ _{Rd,s,l,x} [kN]	HBC-50/30-N M16 8.8F	-	2.4	4.6
		HBC-50/30-N M20 8.8F	-	2.4	4.6	

¹⁾ Internal technical data





Steel resistance values under tension and perpendicular shear load - channel bolts

Static/ quasi-static

HBC channel bolt	diameter		_		M10	M12	M16	M20
Steel failure								
				4.6	23.2		-	
			HBC-40/22	8.8	_	67.4	125.6	
				A4-70	20.5	59.0	91.0	-
		N _{Rk,s}	HBC-40/22-N	8.8	-	-	125.6	-
	Characteristic tension resistance	[kN]		4.6			_	
1			HBC-50/30	8.8		67.4	125.6	147.1
				A4-70	-	59.0	109.9	121.2
			HBC-50/30-N	8.8		_	125.6	186.6
				4.6	11.6		-	
	Design tension resistance		HBC-40/22	8.8	-	44.9	83.7	-
				A4-70	10.9	31.6	47.7	-
		N _{Rd,s} [kN]	HBC-40/22-N	8.8		-	83.7	-
				4.6			_	
			HBC-50/30	8.8	_	44.9	83.7	98.1
				A4-70	_	31.6	58.8	64.8
			HBC-50/30-N	8.8		-	83.7	124.4
			HBC-40/22	4.6	13.9		-	
				8.8	23.2	33.7	62.8	_
				A4-70	24.4	35.4	65.9	
	Characteristic shear resistance	$V_{Rk,s}$	HBC-40/22-N	8.8		-	62.8	-
	characteristic shear resistance	[kN]		4.6			-	
			HBC-50/30	8.8	_	33.7	62.8	101.7
				A4-70		35.4	65.9	102.9
			HBC-50/30-N	8.8		-	62.8	101.7
				4.6	8.3		-	
			HBC-40/22	8.8	18.6	26.9	50.2	_
				A4-70	15.6	22.7	42.2	
	Design shear resistance	V _{Rd,s}	HBC-40/22-N	8.8		-	50.2	-
		[kN]		4.6			-	
			HBC-50/30	8.8	_	26.9	50.2	81.4
				A4-70		22.7	42.2	65.9
			HBC-50/30-N	8.8		-	50.2	81.4



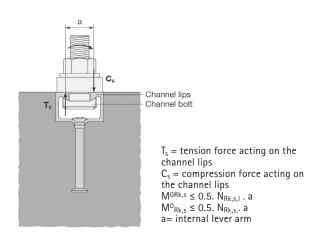


Static/ quasi-static

Steel resistance values under perpendicular shear load with lever arm – channel bolts

HBC channel	HBC channel					M12	M16	M20	
Steel failure									
Characteristic flexural resistance				4.6	29.9 ¹⁾		-		
	Characteristic	NAO.	HBC-40/22 HBC-50/30	8.8	59.8	104.8	266.4	538.7	
	flexural	M ⁰ _{Rk,s} [Nm]	HBC-50/30	A4-70	52.3	91.7	233.1	454.4	
	[IVIII]	HBC-40/22-N	8.8	-	-	266.4	-		
ing I	normal I		HBC-50/30-N	8.8	-	-	266.4	538.7	
				4.6	17.9 ¹⁾		-		
	D :	N.40		8.8	47.8	83.8	213.1	430.9	
·	Design flexural resistance	M ⁰ _{Rd,s} [Nm]		A4-70	33.5	58.8	149.4	291.3	
	resistance	[IVIII]	HBC-40/22-N	8.8	-	-	213.1	-	
Lever arm			HBC-50/30-N	8.8	-	-	213.1	430.9	
	Lover orm	a	HBC-40/22-N	40/22	24.3	25.7	27.3	-	
	[mm]	HBC-50/30-N	50/30	-	29.9	31.7	33.9		

¹⁾Not applicable for HBC-50/30



Displacements under perpendicular shear load

PEC-TA-CE		40/22	50/30	52/34
Shear load	V _y [kN]	10.3	16.0	28.4
Short-term displacement 1)	$\delta_{V0,y}$ [mm]	2.1	2.6	3.7
Long-term displacement 1)	$\delta_{V\infty,y}$ [mm]	3.1	3.9	5.5
Shear load	V _x [kN]	5.8	7.9	7.9
Short-term displacement 1)	$\delta_{V0,x}$ [mm]	0.2	1.4	1.4
Long-term displacement 1)	δ _{V∞,x} [mm]	0.4	2.0	2.0

¹⁾ Displacements in midspan of the anchor channel, including slip of channel bolt, deformation of channel lips and slip of the anchor channel in concrete



Concrete resistance values under static tension and perpendicular shear loads



quasi-static

Concrete resistance values under tension load - anchor channel

PEC-TA-CE						50/30		52/34	
Type of anchor (I-Anchor or R-Round Anchor)						- 1	R	- 1	R
Pullout failure									
	Characteristic resistance C12/15	$N_{Rk,p}$	27.0	13.6	33.8	21.2	68.4	33.2	
	Characteristic resistance C12/15	in uncracked concrete	[kN]	37.8	19.0	47.3	29.7	95.8	46.5
	Design resistance in crack	$N_{Rd,p}$	18.0	9.1	22.5	14.1	45.6	22.1	
	Design resistance in uncra	[kN]	25.2	12.7	31.5	19.8	63.9	31.0	
Amplification factor for other concrete grades			Ψс	$\psi_c = \frac{f_{c,specified}}{12 \text{ (MPa)}}$					
Concrete cone failu	re								
	Product factor k ₁ for	cracked concrete	k _{cr,N}	7	.9	8.1		8	.7
	characteristic resistance uncracked concrete		k _{ucr,N}	11.2		11.6		12.4	
Splitting									
	Characteristic edge distance		c _{cr,sp} [mm]	237		28	32	40	65
Characteristic spacing		S _{cr,sp} [mm]			2.0 ·	C _{cr,sp}			

Concrete resistance values under perpendicular shear load - anchor channel

PEC-TA-CE			40/22	50/30	52/34	
Pry out fail	ıre					
Product factor		k ₈	2.0			
Concrete ed	ge failure					
→	Cracked Concrete		k _{cr,V}	7.5		
K			k _{ucr,V}		10.5	

Interaction factors for combined tension and shear loads

PEC-TA-CE	40/22	50/30	52/34				
Steel failure: Local failure by flexure of channel lips and failure by flexure of channel							
Product factor	k ₁₃		1.0 ¹⁾				
Steel failure: Failure of anchor and connection between anchor and channel							
Product factor	k ₁₄		1.0 ²⁾				

 $^{^{1)}}$ k_{13} can be taken as 2.0 if $V_{Rd,s,l}$ is limited to $N_{Rd,s,l}$ $^{2)}$ k_{14} can be taken as 2.0 if $max(V_{Rd,s,a};\,V_{Rd,s,c})$ is limited to $min(N_{Rd,s,a};\,N_{Rd,s,c})$



Fire resistance



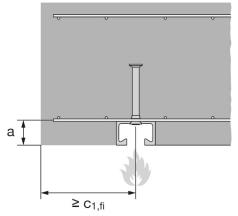
Steel resistance values under fire exposure, tension and perpendicular shear load

Fire resistance

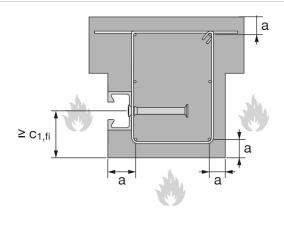
Channel bolt diameter (Only H	BC-X T-bolt)	M10	M12	≥ M16					
Steel failure of anchor, connection between anchor and channel, local flexure of channel lip									
	PEC-TA-CE 40/22	R60	$\begin{array}{c} N_{Rk,s,fi} \\ = \\ V_{Rk,s,fi} \\ or \\ N_{Rd,s,fi} \\ = \end{array}$		1.7	3.5			
		R90			1.2	2.2			
Characteristic and design		R120			0.9	1.5			
resistance in cracked concrete	PEC-TA-CE 50/30	R60		[kN]		3.8	3.9		
C20/25		R90			_	2.5	2.9		
	PEC-TA-CE 52/34	R120	$V_{Rd,s,fi}$		_	1.9	2.4		

Minimum axis distance of reinforcement

PEC-TA-CE			40/22	50/30	52/34
Concrete cover	R60	u [mm]	35	50	50
	R90		45	50	
	R120		55		



Fire exposure from one side only $c_{1,\;fi}=2\;x\;h_{ef}$



Fire exposure from more than one side $c_{1, fi} = max (2 \times h_{ef}; 300mm)$



Combination of anchor channels and channel bolts under fatigue tension load

PEC-TA-CE			Channel bolt			
Anchor channel						
Channel profile	Anchor type	Corrosion protection	Channel bolt	Diameter	Steel grade	Corrosion protection
PEC-TA-CE 52/34	D	Е	HBC-50/30	M16	0.0	G
rec-1A-CE 52/34	52/34 N F		пвс-50/30	M20	8.8	F





Resistance values under fatigue tension load – steel failure after n load cycles without static preload ($N_{Ed} = 0$) (Design method I according to EOTA TR 050)

PEC-TA-CE	52/34		
Steel failure	$\Delta N_{Rk,s,0,n}$ [kN]		
	≤ 10⁴	24.3	
	≤ 10 ⁵	12.5	
Characteristic resistance under fatigue tension load after n load cycles without static preload $(N_{Ed}=0)$	≤ 10 ⁶	7.1	
	≤ 2 · 10 ⁶	6.4	
	≤ 5 · 10 ⁶	5.9	
	≤ 108	5.7	
	> 108	5.5	

Reduction factor $\eta_{c,fat}$ of characteristic fatigue resistance – concrete failure after n load cycles without static preload (N_{Ed} = 0) (Design method I according to EOTA TR 050)

PEC-TA-CE	52/34	
Pull-out and concrete cone failure	n	$\eta_{c,fat}$ [-]
Reduction factor after n load cycles without	≤ 10 ⁴	0.736
static preload ($N_{Ed} = 0$) for:	≤ 10 ⁵	0.665
$\Delta N_{Rk,p,0,n} = \eta_{c,fat} \cdot N_{Rk,p}$	≤ 10 ⁶	0.600
$\Delta N_{Rk,c,0,n} = \eta_{c,fat} \cdot N_{Rk,c}$	≤ 2 · 10 ⁶	0.582
	≤ 5 · 10 ⁶	0.559
with $N_{Rk,p}$ calculated according to page 12 and $N_{Rk,p}$ calculated according to	≤ 6 · 10 ⁷	0.500
EOTA TR047, March 2018 or EN 1992-4: 2018	> 6 · 10 ⁷	0.500

Resistance values under fatigue tension load – steel failure with n $\rightarrow \infty$ load cycles without static preload (N_{Ed} = 0) (Design method II according to EOTA TR 050)

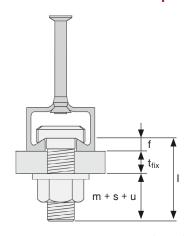
PEC-TA-CE	52/34
Steel failure	$\Delta N_{Rk,s,0,\infty}$ [kN]
Characteristic fatigue limit resistance (n $\longrightarrow \infty$) without static preload (N _{Ed} = 0)	5.5



Reduction factor $\eta_{\text{c,fat}}$ of characteristic fatigue limit resistance – concrete failure with n \rightarrow ∞ load cycles without static preload (N_{Ed} = 0) (Design method II according to EOTA TR 050)

PEC-TA-CE	52/34
Pull-out and concrete cone failure	$\eta_{c,fat}$ [-]
Reduction factor for fatigue limit resistance	
(n $\longrightarrow \infty$) without static preload (N _{Ed} = 0) for:	
$\begin{split} \Delta N_{Rk,p,0,n} &= \eta_{c,fat} \cdot N_{Rk,p} \\ \Delta N_{Rk,c,0,n} &= \eta_{c,fat} \cdot N_{Rk,c} \end{split}$	0.5
with $N_{Rk,p}$ calculated according to page 13 and $N_{Rk,c}$ calculated according to EOTA TR047, March 2018 or EN 1992-4: 2018	

Determination of required T-Bolt length



Required T-Bolt length $I = t_{fix} + f + (m+s+u)$

Profile	Production	Height of channel lip (f)	T–Bolt type	m+s+u [mm]			
		[mm]		M10	M12	M16	M20
PEC-TA-CE 40/22	Hot-rolled	6	HBC-40/22	13.9	17.3	21.8	-
PEC-TA-CE 40/22	Hot-rolled	6	HBC-40/22-N	-	-	21.8	-
PEC-TA-CE 50/30	Hot-rolled	8	HBC-50/30	-	17.3	21.8	27.0
PEC-TA-CE 50/30	Hot-rolled	8	HBC-50/30-N	-	-	21.8	27.0
PEC-TA-CE 52/34	Hot-rolled	11.5	HBC-50/30	-	17.3	21.8	27.0

= nominal length of channel bolt

= fastenable thickness (Thickness of the attached part)

f = height of channel lip

m = thickness of the nut (ISO 4032)

s = thickness of the washer u = channel bolt projection

Note: Round the bolt length to the nearest standard channel bolt length.



Installation instructions

Installation instructions

Installation instructions for anchor channels PEC-TA hot-rolled

- 1) Correct selection of anchor channel in accordance with the design specification.
- 2) If cutting of the anchor channel is necessary, cut the channel with an end spacing

x = 25 or 35 mm for round or welded anchors with profile: PEC-TA-CE 40/22 PEC-TA-CE 50/30

x = 35 mm for round anchor with profile PEC-TA-CE 52/34

x = 25 mm for welded I-anchors with profile PEC-TA-CE 52/34

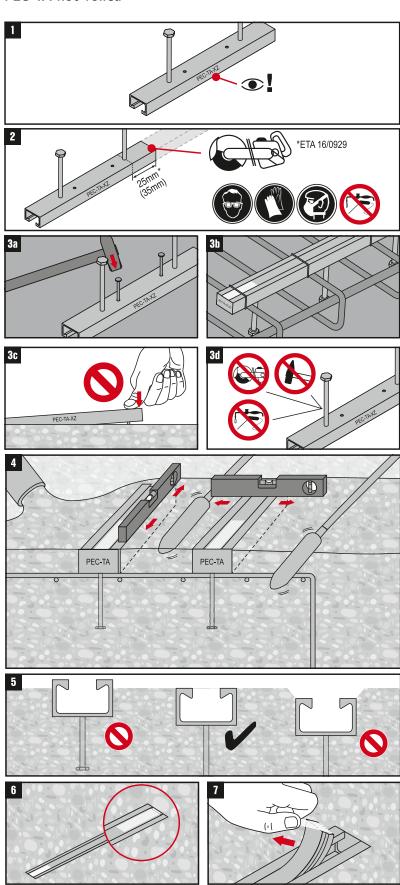
Minimum two anchors per channel!

- 3) Position the anchor channel such that the channel lips will be flush with the surface of the concrete. Secure anchor channels to formwork (3a) or adjoining reinforcing steel (3b) with nails, staples, rivets, or wire ties as appropriate. Supports and attachments shall be adequate to ensure that anchor channels remain in position during concrete pouring. Anchor channels shall not be pushed into fresh concrete (3c). Anchors shall not be bent, cut or otherwise modified (3d).
- 4) Anchor channels shall be protected from intrusion of concrete and slurry into the channel during concrete pouring. Place and consolidate concrete around anchor channels to mitigate voids.

Make sure that channels are leveled.

Installed anchor channels must be flush with the concrete surface.

6 and 7) Remove the foam filler after hardening of concrete and striking the formwork.

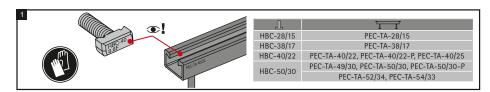




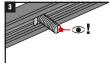
Installation instructions for HBC channel bolts

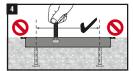
- 1) Select PEC channel bolt type HBC in accordance with the design specification.
- 2) Place the channel bolt in the channel and lock the channel bolt in the channel by turning it 90 degrees.
- 3) Verify alignment of the bolt with the groove.
- 4) Verify that the channel bolt is not located outside of that portion of the channel bounded by the outermost anchors.
- 5) Do not cut channel bolts.
- 6) Install the fixture distinguishing between installation type A and installation type B.
- For installation type A the fixture is in contact with the concrete surface and the channel profile.
- For installation type B the fixture is not in contact with the concrete surface. The fixture is fastened to the anchor channel by a suitable steel element e.g. square plate washer is used to avoid introducing forces into the concrete during application of the installation torque T_{inst}. The steel element shall have a sufficient stiffness to avoid deformation of the channel lips
- 7) Apply the installation torque T_{inst} to the channel bolt with a calibrated torque wrench. Do not exceed the value T_{inst} distinguishing between installation type A and installation type B.

Select the correct installation torque T_{inst} according to material, channel type, channel bolt diameter, and installation type.

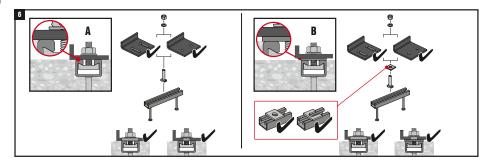


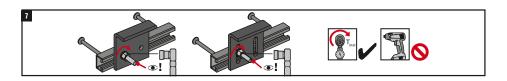












Channel bolt		T _{inst} (Nm] ¹⁾					
		4.6	8.8	A4-50	A4-70		
M8	7		20	7	15		
M10	10	-	40		30		
M12	13		60		50		
M10	15	13	15		22		
M12	25	-	45		50		
M16	40		100		90		
M10	15	13	15	-	22		
M12	25	-	45		50		
M16	30		100		90		
M12	25		45		50		
M16	55		100		130		
M20	55		360		250		
	M8 M10 M12 M10 M12 M16 M10 M12 M16 M12 M16	4.6, 8.8, A4-50, A4-70 M8 7 M10 10 M12 13 M10 15 M12 25 M16 40 M10 15 M12 25 M16 30 M12 25 M16 30 M12 25 M16 55 M20 55	4.6, 8.8, A4-50, A4-70 M8 7 M10 10 - M12 13 M10 15 13 M12 25 M16 40 - M10 15 13 M12 25 M16 30 M12 25 - M16 30 M12 25 - M16 55 M20 55	4.6, 8.8, A4-50, A4-70 4.6 8.8 M8 7 20 M10 10 - 40 M12 13 60 M10 15 13 15 M12 25 - 45 M16 40 100 100 M10 15 13 15 M12 25 45 45 M16 30 100 100 M12 25 - 45 M16 55 100 100 M20 55 360 360	4.6, 8.8, A4-50 M8 7 20 7 M10 10 - 40 M12 13 60 M10 15 13 15 M12 25 - 45 M16 40 100 M10 15 13 15 - M12 25 45 M16 30 100 M12 25 - 45 M16 55 100 M20 55 360		

 $^{^{1)}}T_{inst}$ must not be exceeded

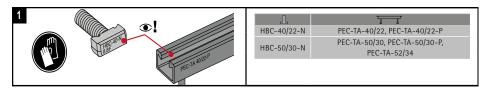


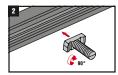
Installation instructions

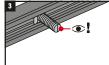
Installation instructions for HBC-N channel bolts

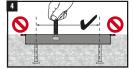
- 1) Select PEC channel bolt type HBC in accordance with the design specification.
- 2) Place the channel bolt in the channel and lock the channel bolt in the channel by turning it 90 degrees.
- 3) Verify alignment of the bolt with the groove.
- 4) Verify that the channel bolt is not located outside of that portion of the channel bounded by the outermost anchors.
- 5) Do not cut channel bolts.
- 6) Install the fixture distinguishing between installation type A and installation type B.
- For installation type A the fixture is in contact with the concrete surface and the channel profile.
- For installation type B the fixture is not in contact with the concrete surface. The fixture is fastened to the anchor channel by a suitable steel element e.g. square plate washer is used to avoid introducing forces into the concrete during application of the installation torque T_{inst}. The steel element shall have a sufficient stiffness to avoid deformation of the channel lips
- 7) Apply the installation torque T_{inst} to the channel bolt with a calibrated torque wrench. Do not exceed the value T_{inst} distinguishing between installation type A and installation type B.

Select the correct installation torque T_{inst} according to material, channel type, channel bolt diameter, and installation type.

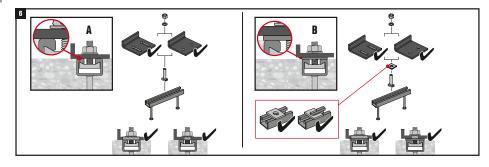


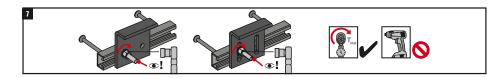












		T _{inst} (Nm] ¹⁾			
Anchor Channel	Channel Bolt	A	B		
		8.8	8.8		
PEC-TA 40/22-P	UDC 40/22 N M10	160	160		
PEC-TA 40/22	HBC-40/22-N M16	60	160		
PEC-TA 50/30-P					
PEC-TA 50/30	HBC-50/30-N M16	185	185		
PEC-TA 52/34					
PEC-TA 50/30-P					
PEC-TA 50/30	HBC-50/30-N M20	320	320		
PEC-TA 52/34					

¹⁾ T_{inst} must not be exceeded





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