Anchoring Technology

Going beyond the limit: NEW PEC-TA-P

Premium Anchor Channel PEC-TA-P hot-rolled

Technical Datasheet December 2020







Selector for premium anchor channels PEC-TA-P hot-rolled

PEC-T	PEC-TA-P hot-rolled anchor channels								
Туре		PEC-TA-C	E 40/22-P	PEC-TA-C	PEC-TA-CE 50/30-P				
Chann	el bolt type	HBC-40/22	HBC-40/22-N	HBC-50/30	HBC-50/30-N				
Chann	el bolt size	M12-M16	M16	M12-M20	M16-M20				
	Cracked concrete	•	•	•	•				
erial	Uncracked concrete				•				
Base material	Normal weight concrete (NWC)	•	•	•	•				
Base	Light weight concrete (LWC)								
	Reinforced/unreinforced	•	•	•	•				
	European Technical Assessment (ETA)		•						
ta	Static 2D	•	•	•					
al da	Static 3D (only HDG)	-	•	-					
Technical data	Seismic	-	-	-	-				
Tec	Fatigue (only HDG)	-	-	•	-				
	Fire	•	-	•	-				
_	Hot-dip galvanized (HDG)	•	•	•					
Specification	Stainless steel A4	•	•	•					
ecifi	Tear-out band	~	~	~	~				
Sp	End caps	~	~	V	✓				
PROFIS	5 Anchor Channel software		•	/					

ETA approved

Internal tests

Free Design-Software

For your individual design we suggest to use our comfortable design software. PROFIS Anchor Channel is a reliable planning tool to optimize the design of anchor channels for each specific construction project. A user-friendly interface allows quick and easy selection of suitable anchor channels and bolts for any type of application.

The calculations are based on the current design code EN 1992-4/EOTA-TR047 and the European Technical Assessment ETA-16/0929.

Benefits



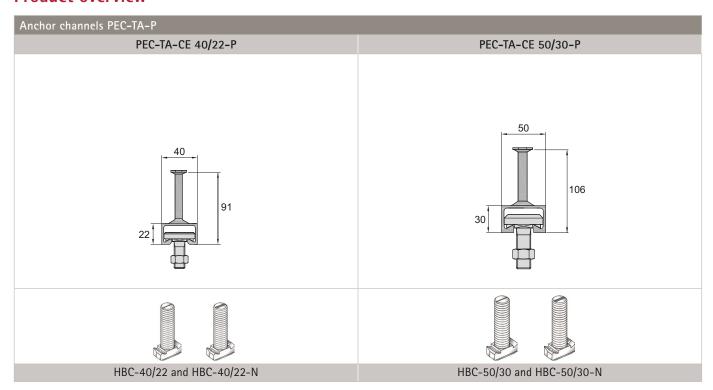
- Fast and efficient planning of fixings with anchor channels
- Clear and concise calculation reports
- Wide range of design parameters
- Functions for automatic planning optimization

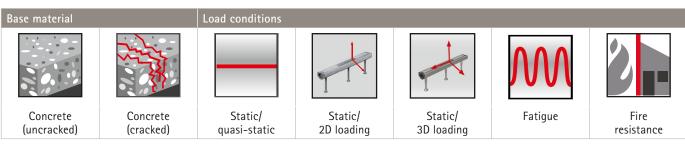
Download PROFIS Anchor Channel Software here:

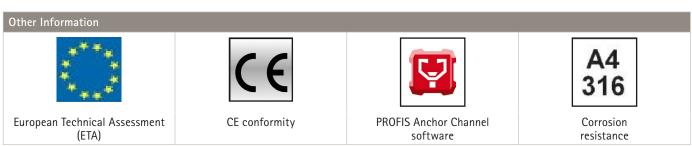
www.pec-europe.com/en/downloads/software.html



Product overview







Approvals

Description	Issuing Authority	No.
European Technical Assessment (ETA) covering 2D, 3D static, fatigue and fire loads.	DIBt Berlin	ETA-16/0929



Product features

Product features



Nomenclature of anchor channels PEC-TA-P hot-rolled

• PEC anchor channel	2 Profile type and size	3 Premium channel	◆ Material finish	6 Foam Filler	6 Anchor channel length [mm]
PEC-TA-CE	40/22	Р	Hot-dip galvanized (FV) or A4 (stainless steel)	PKF	300

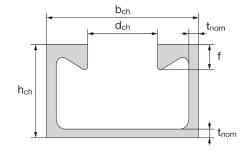
Examples: ① Channel type ② Profile type/size ③ Premium channel ④ Material finish ⑤ Foam Filler ⑥ Length





Channel dimensions hot-rolled profiles

Anchor channel	b _{ch}	h _{ch}	t _{nom}	d _{ch}	f	l _y
			[mm]			[mm ⁴]
PEC-TA-CE 40/22-P	40.1	23.0	2.7	18.0	6.0	21.504
PEC-TA-CE 50/30-P	49.6	30.0	3.2	22.5	8.1	57.781

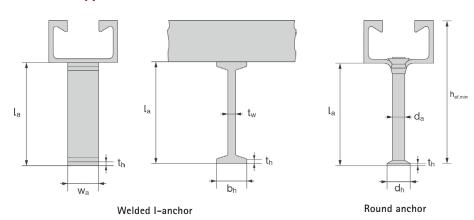


Anchor dimensions (welded I-anchor or round anchor)

Anchor channel		I-anchor 1)			Round anchor						
	min l _a	t _w	b _h	t _h	Wa	A _h	min l _a	d _a	d _h	t _h	A _h
		[mm] [mm²			[mm ²]	[mm]			[mm ²]		
PEC-TA-CE 40/22-P	125.0	6.0	25.0	5.0	20.0	380	70.0	10.0	21.5	2.2	285
PEC-TA-CE 50/30-P	125.0	6.0	25.0	5.0	25.0	475	78.0	11.0	26.0	2.5	436

¹⁾ Available on request . Not on stock.

Anchor types

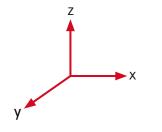


Installation parameters for anchor channels

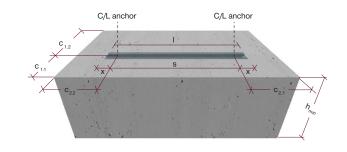
PEC-TA-CE	40/22-P	50/30-P			
Minimum effective embedment depth	h _{ef,min}		91	106	
Minimum anchor spacing S _{min}			50 1)		
Maximum anchor spacing	S _{max}		250		
End spacing	X	[mm]	25 ²⁾		
Minimum channel length	I _{min}		100		
Minimum edge distance (c_{11} , $c_{1,2}$ & c_{21} , c_{22})	C _{min}		50 75		
nimum thickness of concrete member h_{min}			100	120	

 $^{^{1)}}$ $S_{min} = 100$ mm when used in combination with notched bolts

²⁾ The end spacing may be increased from 25 to 35 mm



- X Longitudinal shear -V_x
- Y Perpendicular shear $-\hat{V}_y$
- Z Tension -N





Material of anchor channels and channel bolts

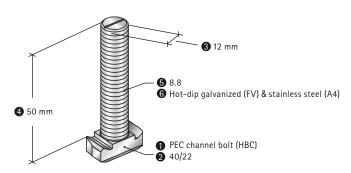
Component		Carbon steel				
	Mechanical properties	Coa	ting	Mechanical properties		
1	2a	2 b	2c	3		
Channel profile	1.0038, 1.0044, 1.0045 according to EN 10025: 2005 1.0976, 1.0979 according to EN 10149: 2013	Hot-dip galvai accord EN ISO 10684:	ling to	1.4362, 1.4401 1.4404, 1.4571, 1.4578 according to EN 10088: 2005		
Anchor	1.0038, 1.0213, 1.0214 according to EN 10025: 2005 1.5523, 1.5535 according to EN 10263: 2002-02	-	Hot-dip galvanized ≥ 50 μm according to EN ISO 10684: 2004/ AC: 2009	1.4362, 1.4401 1.4404, 1.4571, 1.4578 according to EN 10088: 2005 ³⁾		
Channel bolt	Steel grade 4.6 and 8.8 according to EN ISO 898-1: 2013	Electroplated according to EN ISO 4042: 1999	Hot-dip galvanized ≥ 50 μm according to EN ISO 10684: 2004/ AC: 2009	Grade 50 or 70 according to EN ISO 3506: 2009		
Plain washer ¹⁾ according to ISO 7089: 2000 and ISO 7093-1: 2000	Hardness class A ≥ 200 HV	Electroplated according to EN ISO 4042: 1999	Hot-dip galvanized ≥ 50 μm according to EN ISO 10684: 2004/ AC: 2009	1.4401, 1.4404 1.4571, 1.4578 according to EN 10088: 2005		
Hexagonal nut according to ISO 4032: 2012 or DIN 934: 1987-10 ²⁾	Property class 5 or 8 according to EN ISO 898-2: 2012	Electroplated according to EN ISO 4042: 1999	Hot-dip galvanized ≥ 50 μm according to EN ISO 10684: 2004/ AC: 2009	Property class 50, 70 or 80 according to EN ISO 3506: 2009		

Channel bolts nomenclature

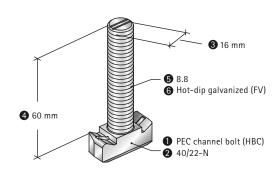
• PEC channel bolt	2 Bolt type	3 Diameter	4 Bolt length [mm]	5 Steel grade	6 Finish or material
НВС	40/22	M12	50	8.8 & A4-70	FV (HDG) or A4 (stainless steel)
НВС	40/22-N	M16	60	8.8	FV (HDG)

Examples: ① Channel bolt ② Bolt type ③ Diameter ④ Bolt length ⑤ Steel grade ⑥ Finish or material

HBC-40/22 M12x50 8.8F (standard bolt)



HBC-40/22-N M16x60 8.8F (notched bolt)



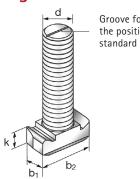
¹⁾ In scope of delivery only for notched bolts
2) Hexagonal nuts according to DIN 934 :1987-10 for channel bolts made from carbon steel (4.6) and stainless steel
3) Anchors made of carbon steel according column 2a may also be used if they are welded and their concrete cover is more than 50mm and the tempering colors are removed



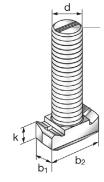
Channel bolt dimensions

Anchor channel	Channel bolt type	Dimensions				
		b ₁	b ₂	k	d	
			[m	m]		
		14.0		10.5	10	
PEC-TA-CE 40/22-P	HBC-40/22	14.0	33.0	11.5	12	
		17.0		11.5	16	
	HBC-40/22-N	17.0	33.0	11.5	16	
		17.0		14.5	12	
	HBC-50/30	17.0	42.0	45.5	16	
PEC-TA-CE 50/30-P		21.0		15.5	20	
	HBC E0/20 N	21.0	42.0	15.5	16	
	HBC-50/30-N	21.0	42.0	15.5	20	

Channel bolts marking



Groove for marking the position of standard bolts



Double groove for marking the position of notched bolts

Scbo

HBC-40/22, HBC-50/30

HBC-40/22-N, HBC-50/30-N

Minimum spacing for channel bolts

Channel bolt			M10	M12	M16	M20
Minimum spacing between channel bolts	S _{cbo,min}	[mm]	50	60	80	100

 s_{cbo} = center to center spacing between channel bolts ($s_{cbo,min}$ = 5d)

Steel grade and corrosion protection

Channel bolt	Carbon	steel 1)	Stainless steel 1)		
Steel grade	4.6	8.8	A4-50	A4-70	
f _{uk} [N/mm ²]	400	800 / 830 ²⁾	500	700	
f _{yk} [N/mm ²]	240	640 / 660 ²⁾	210	450	
Corrosion protection	G 3) F 4)		R ⁵⁾		

¹⁾ Material properties according to table on page 7 ²⁾ Material properties according to EN ISO 898-1 :2013 ³⁾ Electroplated

⁴⁾ Hot-dip galvanized

⁵⁾ Stainless steel



Steel resistance values – static resistance under tension, perpendicular shear and longitudinal shear load



Static/ quasi-static

Steel resistance values under tension load - anchor channel

PEC-TA-CE			40/22-P	50/30-P
Steel fail	lure: Failure of anchor			
	Characteristic resistance	$N_{Rk,s,a}$ [kN]	40.0	57.0
	Design resistance	$N_{Rd,s,a}$ [kN]	22.2	31.7
Steel fail	lure: Failure of connection bet	ween anchor and chai	nnel	
	Characteristic resistance	N _{Rk,s,c} [kN]	39.6	50.6
	Design resistance	N _{Rd,s,c} [kN]	22.0	28.1
Steel fail	lure: Local failure by flexure of	channel lips		
	Characteristic or design spacing of the channel bolts	S _{I,N} [mm]	79.0	98.0
	Characteristic resistance	N ^o _{Rk,s,I} [kN]	47.9	50.5
	Design resistance	N ^o _{Rd,s,l} [kN]	26.6	28.1

Steel resistance values under tension load - channel

PEC-TA-CE		40/22-P	50/30-P	
Steel failure: Fai	lure by flexure of channel			
	Characteristic flexural resistance	M _{Rk,s,flex} [Nm]	1704	3448
	Design flexural resistance	M _{Rd,s,flex} [Nm]	1482	2998

Displacements under tension load

PEC-TA-CE		40/22-P	50/30-P
Tension load	N [kN]	15.3	25.8
Short-term displacement 1)	δ_{N0} [mm]	1.1	1.4
Long-term displacement 1)	$\delta_{N\infty}$ [mm]	2.2	2.8

¹⁾ Displacements in midspan of the anchor channel, including slip of channel bolt, deformation of channel lips, bending of the channel and slip of the anchor channel in concrete



Steel resistance values under perpendicular and longitudinal shear load – anchor channel



Static/ quasi-static

PEC-TA-C			40/22-P	50/30-P
Steel failu	re: Failure of anchor			
	Characteristic resistance	V _{Rk,s,a,y} [kN]	58.1	100.0
-	Characteristic resistance	$V_{Rk,s,a,x}$ [kN]	24.0	34.2
	Design resistance	$V_{Rd,s,a,y}$ [kN]	38.7	66.7
	Design resistance	$V_{Rd,s,a,x}$ [kN]	16.0	22.8
Steel failu	re: Failure of connection betwe	en anchor and cha	nnel	
	Characteristic resistance	V _{Rk,s,c,y} [kN]	58.1	100.0
→	Characteristic resistance	V _{Rk,s,c,x} [kN]	23.8	30.4
ľ	Design resistance	V _{Rd,s,c,y} [kN]	32.3	55.6
	Design resistance	V _{Rd,s,c,x} [kN]	13.2	16.9
Steel failu	re: Local failure by flexure of c	hannel lips		
→	Characteristic or design spacing of the channel bolts for shear	S _{I,V} [mm]	80.0	99.0
	Characteristic resistance	V ⁰ _{Rk,s,l,y} [kN]	55.0	91.7
	Design resistance	V ⁰ _{Rd,s,l,y} [kN]	30.6	50.9

Steel resistance values under perpendicular shear load in direction of the logitudinal axis of the channel – anchor channel

PEC-TA-CE			40/22-P	50/30-P
Steel failure: Connection bet	tween channel lip	s and channel bolt		
		HBC-40/22-N M16 8.8F	12,5	-
Characteristic resistance	V ⁰ _{Rk,s,l,x} [kN]	HBC-50/30-N M16 8.8F		8.3
		HBC-50/30-N M20 8.8F	-	8.3
	V ⁰ _{Rd,s,l,x} HBC [kN] HBC	HBC-40/22-N M16 8.8F	5.0	-
Design resistance		HBC-50/30-N M16 8.8F	-	4.6
		HBC-50/30-N M20 8.8F	-	4.6



Steel resistance values under tension and perpendicular shear load - channel bolts

HBC channel bolt	M10	M12	M16	M20					
Steel failure									
				4.6	23.2		-		
			HBC-40/22	8.8	_	67.4	125.6		
				A4-70	20.5	59.0	91.0	_	
	Characteristic tension resistance	N _{Rk,s}	HBC-40/22-N	8.8	-	-	125.6	-	
	Characteristic tension resistance	[kN]		4.6		-			
			HBC-50/30	8.8		67.4	125.6	147.1	
				A4-70	_	59.0	109.9	121.2	
			HBC-50/30-N	8.8	-	-	125.6	186.6	
				4.6	11.6	-	-	-	
			HBC-40/22	8.8	-	44.9	83.7	-	
	Design tension resistance			A4-70	10.9	31.6	48.7	-	
		N _{Rd,s}	HBC-40/22-N	8.8	-	_	83.7	_	
		[kN]	HBC-50/30	4.6			-		
				8.8	-	44.9	83.7	98.1	
				A4-70	-	31.6	58.8	64.8	
			HBC-50/30-N	8.8	-	-	83.7	124.4	
			HBC-40/22	4.6	13.9		-		
				8.8	23.2	33.7	62.8	_	
				A4-70	24.4	35.4	65.9	_	
	Characteristic shear resistance	V _{Rk,s}	HBC-40/22-N	8.8	-	-	62.8	-	
	Characteristic shear resistance	[kN]		4.6			-		
9			HBC-50/30	8.8	_	33.7	62.8	101.7	
-				A4-70		35.4	65.9	102.9	
			HBC-50/30-N	8.8	-	-	62.8	101.7	
				4.6	8.3		-	ı	
			HBC-40/22	8.8	18.6	26.9	50.2	_	
				A4-70	15.6	22.7	42.2		
	Design shear resistance	V _{Rd,s}	HBC-40/22-N	8.8	-	-	50.2	-	
	z esign sincar resistance	[kN]		4.6			-		
			HBC-50/30	8.8	_	26.9	50.2	81.4	
				A4-70		22.7	42.2	65.9	
			HBC-50/30-N	8.8	-	-	50.2	81.4	



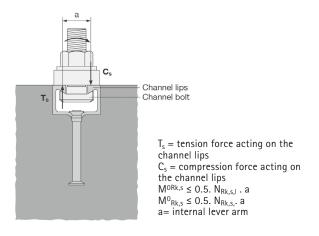


Steel resistance values under perpendicular shear load with lever arm – channel bolts

Static/ quasi-static

HBC channel	HBC channel					M12	M16	M20
Steel failure								
	Characteristic	N 40	LIDO 40/22(N)	4.6	29.9 ¹⁾		-	
	flexural	M ⁰ _{Rk,s} [Nm]	HBC-40/22(-N) HBC-50/30(-N)	8.8	59.8	104.8	266.4	538.7
inner I	resistance	stance	1100-30/30(-11)	A4-70	52.3	91.7	233.1	454.4
			4.6	17.9 ¹⁾		-		
	Design flexural resistance	M ⁰ _{Rd,s} [Nm]	HBC-40/22(-N) HBC-50/30(-N)	8.8	47.8	83.8	213.1	430.9
	resistance	[IVIII]	1100-30/30(-11)	A4-70	33.5	58.8	149.4	291.3
	Internal lever	а	a HBC-40/22(-N)	40/22	24.3	25.7	27.3	
	arm	[mm]	HBC-50/30(-N)	50/30	-	29.9	31.7	33.9

¹⁾ Not applicable for HBC-50/30



Displacements under perpendicular shear load

PEC-TA-CE		40/22-P	50/30-P
Shear load	V _y [kN]	29.0	39.7
Short-term displacement 1)	$\delta_{V0,y}$ [mm]	2.0	2.7
Long-term displacement 1)	δ _{V∞,y} [mm]	3.5	4.0
Shear load	V _x [kN]	5.2	3.3
Short-term displacement 1)	$\delta_{V0,x}$ [mm]	0.1	0.1
Long-term displacement 1)	$\delta_{V_{\infty,X}}[mm]$	0.2	0.2

¹⁾ Displacements in midspan of the anchor channel, including slip of channel bolt, deformation of channel lips and slip of the anchor channel in concrete



Concrete resistance values under static tension and perpendicular shear loads



quasi-static

Concrete resistance values under tension load - anchor channel

PEC-TA-CE	40/22-P		50/30-P				
Type of anchor (I-A	I	R	I	R			
Pullout failure							
A	$N_{Rk,p}$	34.2	25.6	42.8	39.2		
	Characteristic resistance C12/15	in uncracked concrete	[kN]	47.9	35.8	59.9	54.9
	Design resistance in crack	ked concrete C12/15	$N_{Rd,p}$	22.8	17.1	28.5	26.1
	Design resistance in uncra	acked concrete C12/15	[kN]	31.9	23.9	39.9	36.6
	Amplification factor for other concrete grades			$\psi_c = \frac{f_{c,specified}}{_{12 \text{ (MPa)}}}$			
Concrete cone failu	ire						
	Product factor k ₁ for	Product factor k ₁ for		8	.0	8	.2
	characteristic resistance	k _{ucr,N}	11.5		11.7		
Splitting							
Characteristic edge distance		nce	c _{cr,sp} [mm]	27	73	3	18
	Characteristic spacing		S _{er,sp} [mm]		2.0 ·	C _{cr,sp}	

Concrete resistance values under perpendicular shear load - anchor channel

PEC-TA-CE		40/22-P	50/30-P		
Pry out fail	ure				
	Product factor		k ₈	2.	0
Concrete ed	ge failure				
→		cracked concrete	k _{er,V}	7.	5
Z	characteristic resistance uncracked concrete		k _{ucr,V}	10	1.5

Interaction factors for combined tension and shear loads

PEC-TA-CE		40/22-P	50/30-P				
Steel failure: Local failure by flexure of channel lips and failure by flexure of channel							
Product factor	k ₁₃	1.0	O ¹⁾				
Steel failure: Failure of anchor and connection between anchor and channel							
Product factor	k ₁₄	1.0) ²⁾				

 $^{^{1)}}$ k_{13} can be taken as 2.0 if $V_{Rd,s,l}$ is limited to $N_{Rd,s,l}$ $^{2)}$ k_{14} can be taken as 2.0 if $max(V_{Rd,s,a};V_{Rd,s,c})$ is limited to $min(N_{Rd,s,a};N_{Rd,s,c})$



Fire resistance



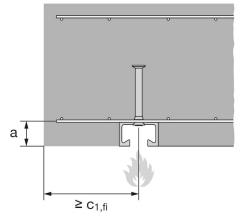
Steel resistance values under fire exposure, tension and perpendicular shear load

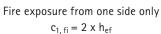
Fire

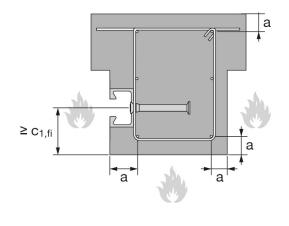
Channel bolt diameter	M10	M12	≥ M16						
Steel failure of anchor, connection between anchor and channel, local flexure of channel lip									
		R60	N _{Rk,s,fi}		1.7	3	.5		
	PEC-TA-CE 40/22-P	R90	$= \\ V_{Rk,s,fi} \\ or \\ N_{Rd,s,fi} \\ = \\ V_{Rd,s,fi}$		1.2	2.2			
Characteristic and design		R120		[kN]	0.9	1	.5		
resistance in cracked concrete		R60				3.8	3.9		
C20/25	PEC-TA-CE 50/30-P	R90			_	2.5	2.9		
		R120			-	1.9	2.4		

Minimum axis distance of reinforcement

PEC-TA-CE		40/22-P	50/30-P	
	R60		35	50
Min axis distance	R90	a [mm]	45	50
	R120	Littiii	55	







Fire exposure from more than one side $c_{1, fi} = max (2 \times h_{ef}; 300mm)$



Combination of anchor channels and channel bolts under fatigue tension load

PEC-TA-CE			Channel bolt			
Anchor channel						
Channel profile	Anchor type	Corrosion protection	Channel bolt	Diameter	Steel grade	Corrosion protection
DEO TA OF 40/00 D	R	F	HBC-40/22	M12	8.8	G
PEC-TA-CE 40/22-P				M16		
DEC TA CE FO/OO D			HBC-50/30	M16		F
PEC-TA-CE 50/30-P				Man		





Resistance values under fatigue tension load – steel failure after n load cycles without static preload ($N_{Ed} = 0$) (Design method I according to EOTA TR 050)

PEC-TA-CE		40/22-P	50/30-P	
Steel failure n		$\Delta N_{Rk,s,0,n}$ [kN]		
	≤ 10⁴	16.4	20.9	
	≤ 10 ⁵	7.7	9.0	
Characteristic resistance under fatigue	≤ 10 ⁶	3.2	4.2	
tension load after n load cycles without static preload	≤ 2 · 10 ⁶	2.6	3.7	
$(N_{Ed} = 0)$	≤ 5 · 10 ⁶	2.2	3.4	
	≤ 10 ⁸	2.0	3.3	
	> 108	1.8	3.2	

Reduction factor $\eta_{c,fat}$ of characteristic fatigue resistance – concrete failure after n load cycles without static preload (N_{Ed} = 0) (Design method I according to EOTA TR 050)

PEC-TA-CE		40/22-P	50/30-P	
Pull-out and concrete cone failure n		η _{c,fat} [-]		
Reduction factor after n load cycles without	≤ 10⁴	0.736		
static preload ($N_{Ed} = 0$) for:	≤ 10 ⁵	0.665		
$\Delta N_{Rk,p,0,n} = \eta_{c,fat} \cdot N_{Rk,p}$	≤ 10 ⁶	0.600		
$\Delta N_{Rk,c,0,n} = \eta_{rfat} \cdot N_{Rk,c}$	≤ 2 · 10 ⁶	0.582		
with N appropriate apporting to page 12	≤ 5 · 10 ⁶	0.5	559	
with $N_{Rk,p}$ calculated according to page 12 and $N_{Rk,c}$ calculated according to	≤ 6 · 10 ⁷	0.500		
EOTA TR047, March 2018 or EN 1992-4: 2018	> 6 · 10 ⁷	0.5	500	

Resistance values under fatigue tension load – steel failure with n $\rightarrow \infty$ load cycles without static preload (N_{Ed} = 0) (Design method II according to EOTA TR 050)

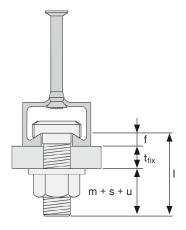
PEC-TA-CE	40/22-P	50/30-P		
Steel failure	$\Delta N_{Rk,s,0}$, ∞ [kN]			
Characteristic fatigue limit resistance (n $\longrightarrow \infty$) without static preload (N _{Ed} = 0)	1.8	3.2		



Reduction factor $\eta_{c,fat}$ of characteristic fatigue limit resistance – concrete failure with n \rightarrow ∞ load cycles without static preload (N_{Ed} = 0) (Design method II according to EOTA TR 050)

PEC-TA-CE	40/22 - P	50/30 - P	52/34-P		
Pull-out and concrete cone failure	η _{c,fat} [-]				
Reduction factor for fatigue limit resistance					
$(n \longrightarrow \infty)$ without static preload $(N_{Ed} = 0)$ for:					
$\Delta N_{Rk,p,0,n} = \eta_{c,fat} \cdot N_{Rk,p}$	0.5				
$\Delta N_{Rk,c,0,n} = \eta_{c,fat} \cdot N_{Rk,c}$					
with $N_{Rk,p}$ calculated according to page 13 and $N_{Rk,c}$ calculated according to EOTA TR047, March 2018 or EN 1992-4: 2018					

Determination of required T-Bolt length



Required T-Bolt length $I = t_{fix} + f + (m+s+u)$

Profile	Height of channel lip (f)	T–Bolt type	m+s+u [mm]			
	[mm]		M10	M12	M16	M20
PEC-TA-CE 40/22-P	6	HBC-40/22	13.9	17.3	21.8	-
PEC-TA-CE 40/22-P	6	HBC-40/22-N	-	-	21.8	-
PEC-TA-CE 50/30-P	8	HBC-50/30	-	17.3	21.8	27.0
PEC-TA-CE 50/30-P	8	HBC-50/30-N	_	_	21.8	27.0

I = nominal length of channel bolt

 t_{fix} = fastenable thickness (Thickness of the attached part)

f = height of channel lip

m = thickness of the nut (ISO 4032)

s = thickness of the washer

u = channel bolt projection

Note: Round the bolt length to the nearest standard channel bolt length.



Installation instructions

Installation instructions

Installation instructions for anchor channels PEC-TA-P hot-rolled

- 1) Correct selection of anchor channel in accordance with the design specification.
- 2) If cutting of the anchor channel is necessary, cut the channel with an end spacing

x = 25 or 35 mm for round or welded anchors with profile:

PEC-TA-CE 40/22-P PEC-TA-CE 50/30-P

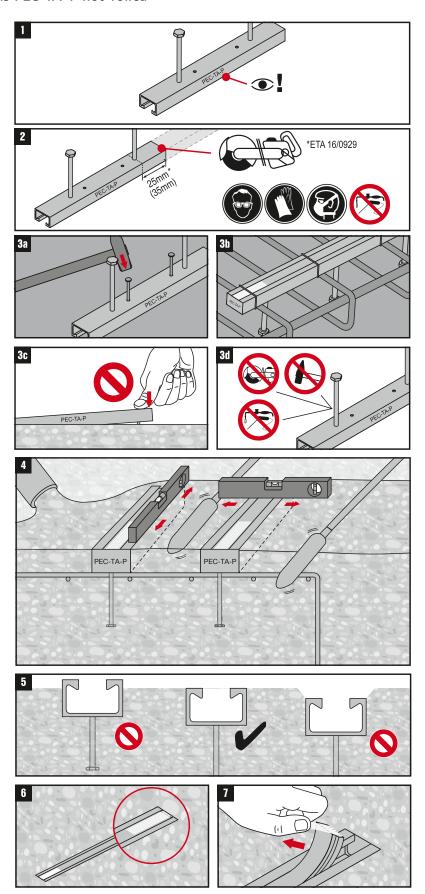
Minimum two anchors per channel!

- 3) Position the anchor channel such that the channel lips will be flush with the surface of the concrete. Secure anchor channels to formwork (3a) or adjoining reinforcing steel (3b) with nails, staples, rivets, or wire ties as appropriate. Supports and attachments shall be adequate to ensure that anchor channels remain in position during concrete pouring. Anchor channels shall not be pushed into fresh concrete (3c). Anchors shall not be bent, cut or otherwise modified (3d).
- 4) Anchor channels shall be protected from intrusion of concrete and slurry into the channel during concrete pouring. Place and consolidate concrete around anchor channels to mitigate voids.

Make sure that channels are leveled.

5) Installed anchor channels must be flush with the concrete surface.

6 and 7) Remove the foam filler after hardening of concrete and striking the formwork.

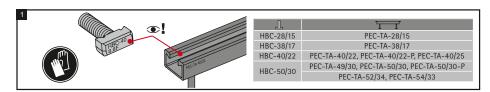




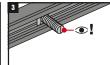
Installation instructions for HBC channel bolts

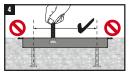
- 1) Select PEC channel bolt type HBC in accordance with the design specification.
- 2) Place the channel bolt in the channel and lock the channel bolt in the channel by turning it 90 degrees.
- 3) Verify alignment of the bolt with the groove.
- 4) Verify that the channel bolt is not located outside of that portion of the channel bounded by the outermost anchors.
- 5) Do not cut channel bolts.
- 6) Install the fixture distinguishing between installation type A and installation type B.
- For installation type A the fixture is in contact with the concrete surface and the channel profile.
- For installation type B the fixture is not in contact with the concrete surface. The fixture is fastened to the anchor channel by suitable steel element e.g. square plate washer is used to avoid introducing forces into the concrete during application of the installation torque T_{inst}. The steel element shall have a sufficient stiffness to avoid deformation of the channel lips
- 7) Apply the installation torque T_{inst} to the channel bolt with a calibrated torque wrench. Do not exceed the value T_{inst} distinguishing between installation type A and installation type B.

Select the correct installation torque T_{inst} according to material, channel type, channel bolt diameter, and installation type.

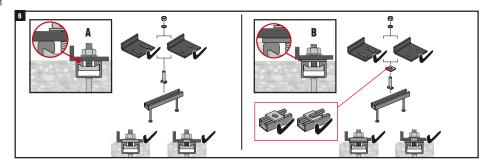


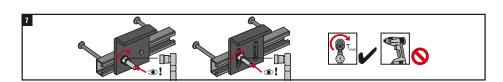












Channel bolt		T _{inst} (Nm] ¹⁾					
		A					
		4.6, 8.8, A4-50, A4-70	4.6	8.8	A4-50	A4-70	
	M8	7		20	7	15	
HBC-28/15	M10	10	-	40		30	
	M12	13		60		50	
	M10	15	13	15		22	
HBC-38/17	M12	25		45		50	
	M16	40	-	100		90	
	M10	15	13	15	-	22	
HBC-40/22	M12	25		45		50	
	M16	30	-	100		90	
HBC-50/30	M12	25		45		50	
	M16	55		100		130	
	M20	55		360		250	

 $^{^{1)}}T_{inst}$ must not be exceeded

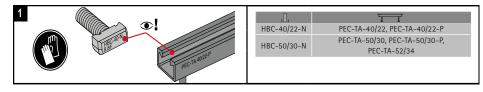


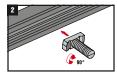
Installation instructions

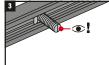
Installation instructions for HBC-N channel bolts

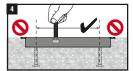
- 1) Select PEC channel bolt type HBC in accordance with the design specification.
- 2) Place the channel bolt in the channel and lock the channel bolt in the channel by turning it 90 degrees.
- 3) Verify alignment of the bolt with the groove.
- 4) Verify that the channel bolt is not located outside of that portion of the channel bounded by the outermost anchors.
- 5) Do not cut channel bolts.
- 6) Install the fixture distinguishing between installation type A and installation type B.
- For installation type A the fixture is in contact with the concrete surface and the channel profile.
- For installation type B the fixture is not in contact with the concrete surface. The fixture is fastened to the anchor channel by suitable steel element e.g. square plate washer is used to avoid introducing forces into the concrete during application of the installation torque T_{inst}. The steel element shall have a sufficient stiffness to avoid deformation of the channel lips
- 7) Apply the installation torque T_{inst} to the channel bolt with a calibrated torque wrench. Do not exceed the value T_{inst} distinguishing between installation type A and installation type B.

Select the correct installation torque T_{inst} according to material, channel type, channel bolt diameter, and installation type.

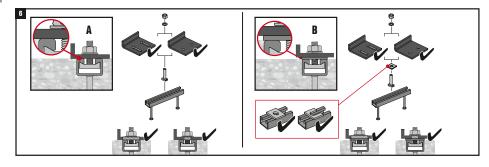


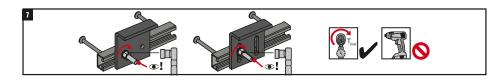












		T _{inst} (Nm] ¹⁾			
Anchor Channel	Channel Bolt	A	B		
		8.8	8.8		
PEC-TA 40/22-P	UDC 40/22 N M46	160	160		
PEC-TA 40/22	HBC-40/22-N M16	60	160		
PEC-TA 50/30-P					
PEC-TA 50/30	HBC-50/30-N M16	185	185		
PEC-TA 52/34					
PEC-TA 50/30-P					
PEC-TA 50/30	HBC-50/30-N M20	320	320		
PEC-TA 52/34					

¹⁾ T_{inst} must not be exceeded





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